SALT LAKE COMMUNITY COLLEGE
REDWOOD ROAD CAMPUS
BUS STOP CANOPY

4600 SOUTH REDWOOD ROAD
SALT LAKE CITY, UTAH 84123

STATE OF UTAH
DEPARTMENT OF ADMINISTRATIVE SERVICES
DIVISION OF FACILITIES CONSTRUCTION AND MANAGEMENT
4110 State Office Building / SLC, Utah 84114 / (801) 538-3018

DFCM PROJECT NO. 06305660

THOMAS AND KOLKMAN
ENGINEERING INC.
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ABBREVIATIONS

ABOVE FINISHED FLOOR	AFF
ALTERNATE	ALT.
ALUMINUM	AL.
AMERICAN CONCRETE INSTITUTE	ACI
AMERICAN NATIONAL STANDARDS INSTITUTE	ANSI
AMERICAN SOCIETY OF TESTING & MATERIALS	ASTM
AMERICAN WELDING SOCIETY	AWS
ANCHOR BOLT	A.B.
AND	&
ANGLE	L
APPROVED	APPD.
APPROXIMATE	APPROX.
ARCHITECT OR ARCHITECTURAL	ARCH.
ARCHITECTURAL CONCRETE	AC
AVERAGE	AVG.
AT	@
BEAM	BM
BLOCKING	BLKG.
BOTTOM	BOT.
BRACKET	BRKT.
BUILDING	BLDG.
CALCIUM SILICATE MASONRY UNIT	CSMU
CATCH BASIN	C.B.
CENTER	CTR.
CENTERLINE	CL
CENTER TO CENTER	C TO C
CHANNEL	[
CLEANOUT	C.O.
COLUMN	COL.
COMPOSITION	COMP.
CONCRETE MASONRY UNIT	C.M.U.
CONCRETE REINFORCING STEEL INSTITUTE	CRSI
CONSTRUCTION	CONST.
CONTINUOUS	CONT.
COUNTERSINK	CSK.
CUBIC	CU.
CUBIC FOOT	CU. FT.
CUBIC FEET PER MINUTE	CFM
CUBIC INCH	CU. IN.
CUBIC YARD	CU. YD.
DEPARTMENT	DEPT.
DIAGONAL	DIAG.
DIAMETER	ø
DIMENSION	DIM.
DOOR	DR
DOUBLE	DBL.
DRAWING	DWG.
ELECTRICAL	ELEC.
ELECTRIC WATER COOLER	EWC
ELEVATION	EL.
EQUIPMENT	EQUIP.
EXPANSION JOINT	EJ / EXP. JT.
EXTERIOR INSULATION FINISH SYSTEM	EIFS
FAR SIDE	F.S.
FEET OR FOOT	FT. or '
FIELD VERIFY	F.V.
FINISH FLOOR	F.F.
FIRE EXTINGUISHER CABINET	F.E.C.
FIRE HOSE CABINET	F.H.C.
FIRE HYDRANT	F.H.
FIRE RETARDANT TREATED	FRT
FLOOR DRAIN	FD
FLUORESCENT	FLUOR.
FOOTING	FTG.
FOUNDATION	FND.
GALVANIZED	GALV.
GAGE OR GAUGE	GA.
GLAZED STRUCTURAL UNIT	GSU
GYPSUM BOARD	GYP. BD.
HARDWARE	HDW.
HEIGHT	HGT.
HIGH STRENGTH BOLT	HSB
HORIZONTAL	HORIZ.
INCH	"
INFORMATION	INFO.
INSIDE DIAMETER	I.D.
INTERMEDIATE	INTER.
KIP (1,000 LB.)	K
LABORATORY	LAB.
MANUFACTURER	MFG.
MAXIMUM	MAX.
MECHANICAL	MECH.
MINIMUM	MIN.
MISCELLANEOUS	MISC.
NATIONAL BOARD OF FIRE UNDERWRITERS	NBFU
NATIONAL ELECTRICAL CODE	NEC
NATIONAL ELECTRICAL MANUFACTURERS ASSOC.	NEMA
NEAR SIDE	N.S.
NOT IN CONTRACT	N.I.C.
NOT TO SCALE	NTS
NUMBER	NO. or #
ON CENTER	O.C.
OPENING	OPNG.
OPPOSITE	OPP.
OUTSIDE DIAMETER	O.D.
PENNY	d
PER	/
PERPENDICULAR	PERP.
PHASE	ø
POUND	LB. or #
PREFABRICATED	PREFAB.
PROPERTY LINE	P/L
RADIUS	R
REINFORCING	REINF.
REQUIRED	REQ'D.
REVISION	REV.
ROD & SHELF	R&S
ROOF DRAIN	RD
ROOM	RM
ROUND	RD. or ø
SHEET	SHT.
SIMILAR	SIM.
SOUND ATTENUATION BLANKET	SAB
SPECIFICATION	SPEC.
SQUARE	SQ. or
SYMMETRICAL	SYM.
SYNTHETIC STUCCO EXTERIOR INSULATION SYSTEM	SSEIS
TOP OF MASONRY	T.O.M.
TOP BACK OF CURB	T.B.C.
TOP OF LANDSCAPING	T.L.
TOP OF WALK	T.W.
TOP OF STEEL	TOS
TOP OF WALL	TOW
TYPICAL	TYP.
UNLESS NOTED OTHERWISE	U.N.O.
VERTICAL	VERT.
VINYL WALL COVERING	VWC.
WELDED WIRE FABRIC	WWF
WITH	W/
WITHOUT	W/O

MATERIAL DESIGNATIONS

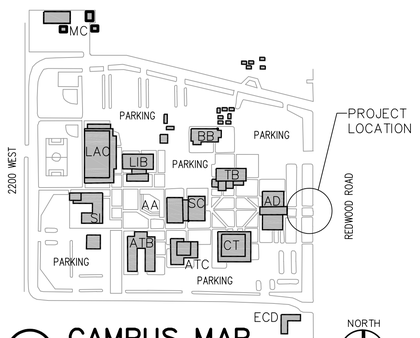
- EARTH
- POROUS FILL
- ASPHALT
- CONCRETE
- CONCRETE MASONRY UNITS
- BRICK
- CAST STONE
- CERAMIC TILE
- WOOD (FINISH)
- WOOD (STUDS, NAILERS)
- WOOD (BLOCKING)
- PLYWOOD
- BATT INSULATION
- RIGID INSULATION
- PLASTER
- ACOUSTIC TILE
- GYPSUM BOARD
- GLASS
- STEEL
- PARTICLE BOARD
- RIGID INSULATION

GRAPHIC SYMBOLS

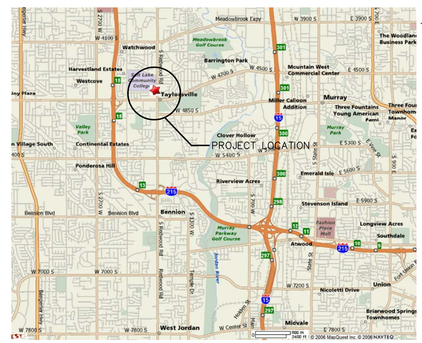
- D2 / AE-50 — DETAIL/WALL or SECTION NUMBER / SHEET NUMBER
- 102 — DOOR NUMBER
- 5 — WINDOW NUMBER
- OFFICE / 100 — ROOM NAME / ROOM NUMBER
- 3 — REVISION NUMBER
- DETAIL, WALL or SECTION NUMBER / SHEET NUMBER
- A — GRID REFERENCE
- PLAN NORTH / ACTUAL NORTH — REFERENCE NORTH (PLANS)
- ELEVATION REFERENCE
- 3 — KEYED NOTE NUMBER
- A / C / B3 / AE201 / B / D — INTERIOR ELEVATION MARKER
- A — A — WALL TYPES

DRAWING SCHEDULE

SHEET #	DRAWING TITLE
GI-001	TITLE SHEET
GI-002	GENERAL INFORMATION
SJ-001	GENERAL STRUCTURAL NOTES
S-101	FOOTING, FOUNDATION AND ROOF PLANS
S-501	ROOF DETAILS
AE-101	PLANS & KEYED NOTES
AE-201	ELEVATIONS & KEYED NOTES
AE-301	SECTION
AE-302	SECTION
AE-501	DETAILS
AE-502	DETAILS
E-101	ELECTRICAL PLANS AND KEYED NOTES



A4 CAMPUS MAP
NOT TO SCALE



A5 LOCATION MAP
NOT TO SCALE

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PROFESSIONAL SEAL

CONSULTANTS

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BHB CONSULTING

ELECTRICAL ENGINEER
THOMAS & KOLKMAN ENGINEERING

SALT LAKE COMMUNITY COLLEGE
REDWOOD ROAD CAMPUS
BUS STOP CANOPY

4600 SOUTH REDWOOD ROAD
SALT LAKE CITY, UT 84123

MARK	DATE	DESCRIPTION
	10.11.06	CONSTRUCTION DOCUMENTS

DFCM PROJECT NO:	06305660
ARCH. PROJECT NO:	05-08
CAD DWG FILE:	GI-002.DWG
DRAWN BY:	JBE
CHECKED BY:	SPE
DESIGNED BY:	SPE

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SHEET TITLE
GENERAL INFORMATION

GI-002

GENERAL STRUCTURAL NOTES

GENERAL

1. The structural notes are intended to complement the project specifications. Specific notes and details in the drawings shall govern over the structural notes and typical details.
2. Typical details and sections shall apply where specific details are not shown.
3. The contractor shall verify all site conditions and dimensions. If actual conditions differ from those shown in the contract drawings, the contractor shall immediately notify the architect/engineer before proceeding with the fabrication or construction of any effected elements.
4. Omissions or conflicts between the contract drawings and/or specifications shall be brought to the attention of the architect/engineer before proceeding with any work involved. In case of conflict, follow the most stringent requirement as directed by the architect/engineer at no additional cost to the owner.
5. The contractor shall submit a written request to the architect/engineer before proceeding with any changes, substitutions or modifications. Any work done by the contractor before receiving written approval will be at the contractor's risk.
6. The contractor shall coordinate with all trades any items that are to be integrated into the structural system such as openings, penetrations, mechanical and electrical equipment, etc. Sizes and locations of mechanical and other equipment that differs from those shown on the contract drawings shall be reported to the architect/engineer.
7. The contractor shall provide adequate shoring and bracing as required for his method of erection. Shoring and bracing shall remain in place until final connections for the permanent members are completed. The building shall not be considered stable until all connections are completed. Walls shall not be considered self-supporting and shall be braced until the floor/roof system is completed.
8. Site observations by BHB Consulting Engineers, P.C.'s field representative shall not be construed as approval of construction procedures nor special inspection.
9. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in the architectural, structural and/or other consultants' drawings. The structural drawings shall be used in conjunction with the architectural and other consultant's drawings. Some dimensions and elements such as elevations, depressions, slopes, mechanical housekeeping pads, etc. are not shown in the structural drawings. All dimensions shown on structural drawings shall be verified by contractor with architectural, mechanical and electrical drawings.
10. Review of shop drawing submittals by BHB Consulting Engineers, P.C. is for general compliance only and is not intended for approval. The shop drawing review shall not relieve the contractor from the responsibility of completing the project according to the contract documents.
11. Shop drawings made from reproductions of the contract drawings will be rejected unless the contractor signs a release agreement prior to the shop drawings being reviewed.
12. Only an authorized representative of BHB Consulting Engineers, P.C. may make changes to these contract drawings. BHB Consulting Engineers, P.C. shall not be held responsible or liable for any claims arising directly or indirectly from changes made without written authorization by an authorized representative of BHB Consulting Engineers, P.C.

BASIS OF DESIGN

1. Governing Building Code — International Building Code 2003
2. Roof Snow Load
   - a. Ground Snow Load — P_g = 43 psf
   - b. Snow Importance Factor — I_s = 1.0
   - c. Snow Exposure Coefficient — C_e = 1.0
   - d. Thermal Exposure Coefficient — C_t = 1.2
   - e. Roof Snow Load — $P_f = 0.7 * C_e * C_t * I_s * P_g$ = 36 psf plus Snow Drift
3. Seismic Loads
   - a. Short Period Mapped Spectral Acceleration — S_S = 1.43
   - b. Soil Site Class — D
   - c. Short Period Site Coefficient — F_a = 1.0
   - d. 5% Damped Design Spectral Response Acceleration — $S_{DS} = 2/3 * F_a * S_S$
   - e. Seismic Importance Factor — I_E = 1.00
   - f. Response Modification Coefficient — R = 2.5
   - g. Seismic Response Coefficient — $C_S = S_{DS} * I_E / R$
   - h. W — Dead Loads of Structure
   - i. Building Seismic Design Category — D
   - j. System Overstrength Factor — 3
   - k. Deflection Amplification Factor — 3
   - l. Base Shear — V= $C_S * W$ = 0.38 W (Strength Design)
4. Wind Loads
   - a. Wind Velocity (3 Second Gust) — 90 mph
   - b. Exposure Type — C
   - c. Wind Importance Factor — 1.00

FOUNDATION

1. Soils Investigation Report: None
2. Soil bearing pressure: 1500 psf - Assumed by owner.
3. Frost Protection: 30 inches minimum.
4. Clear excavations of debris and loose soil prior to placing footings. All footings shall bear on undisturbed natural sub-grade or engineered compacted fill as noted in these drawings.

EARTHWORK

1. Clearing: The entire building area shall be scraped to remove the top 4 inches of soil, including all vegetation and debris.
2. Proof rolling: The natural undisturbed soil below all footings shall be proof rolled prior to placing concrete. Remove all soft spots and replace with compacted structural fill.
3. Compacted structural fill: All fill material shall be a well-graded granular material with a maximum size less than 4 inches and with not more than 10 percent passing a No. 200 sieve. It shall be compacted to 95 percent of the maximum laboratory density as determined by ASTM D 1557. All fill shall be tested.

CONCRETE

1. Materials, unless noted otherwise:
   - a. Normal weight aggregates — ASTM C 33
   - b. Reinforcing Steel — ASTM 615 Grade 60 (Fy = 60 ksi) Use Grade 40 (Fy = 40 ksi) for field bent dowels with spacings indicated reduced by 1/3.
   - c. Deformed Bar Anchors (DBA) — ASTM A496
   - d. Headed Stud Anchors (HSA) — ASTM A108
   - e. Anchor Rods — ASTM F1554 Grade 36 with ASTM A563 heavy hex nuts with hardened washers
   - f. Admixtures:
     - i. Air-entraining admixtures comply with ASTM C 260 (when used).
     - ii. Calcium chloride shall not be added to the concrete mix.
   - g. Type I/II cement complying with ASTM C-150 shall be used for all concrete.
   - h. The water/cement ratios shall meet the requirements of ACI 318.
   - i. Provide air entraining as recommended by ACI 318.
   - j. No aluminum conduit or product containing aluminum or any other material injurious to concrete shall be embedded in concrete.
2. Compressive strengths of concrete at 28 days shall be as follows:
   - a. Footings ..... 3,000 psi
   - b. Walls ..... 3,000 psi
   - c. Columns ..... 4,000 psi
   - d. All Site Concrete ..... 4,000 psi
3. Only one grade or type of concrete shall be poured on the site at any given time.
4. The contractor shall be responsible for the design, detailing, care, placement and removal of all formwork and shores.
   - a. Supporting forms and shoring shall not be removed until structural members have acquired sufficient strength to safely support their own weight and any construction load to which they may be subjected. In no case, however, shall forms and shoring be removed in less than 24 hours after concrete placement.
5. Reinforcement shall have the following concrete cover:
   Cast-in-place Concrete ..... Clear Cover
   - a. Cast against and permanently exposed to earth ..... 3"
   - b. Formed concrete exposed to earth or weather:
     - #6 thru #18 bars ..... 2"
     - #5 and smaller bars ..... 1-1/2"
   - c. Concrete not exposed to weather or in contact with ground:
     - Slabs, Walls, Joists; #11 bars and smaller ..... 3/4"
     - Beams, Columns: Primary Reinf., Ties, Stirrups, Spirals ..... 1-1/2"
6. Construction Joints and Control Joints:
   - a. Provide a formed and beveled 2 x 4 x continuous keyway in all horizontal and vertical construction joints including between top of footing and foundation walls, unless noted otherwise. In addition, all joints shall be intentionally roughened to a full amplitude of approximately 1/4 inch.
   - b. Control joints shall be installed in slabs on grade so the length to width ratio of the slab is no more than 1.25:1. Control joints shall be completed within 12 hours of concrete placement. Control joints may be installed by:
     - i. Saw cut a depth of 1/4 the thickness of the slab
     - ii. Tooled joints a depth of 1/4 the thickness of the slab
   - c. Install construction or control joints in slabs on grade at a spacing not to exceed 30 times the slab thickness in any direction for unreinforced slabs and 75 times the slab thickness in any direction for reinforced slabs, unless noted otherwise. Construction joints shall not exceed a distance of 125'-0" o.c. in any direction.
8. Construction
   - a. Use chairs or other support devices recommended by the CRSI to support and tie reinforcement bars and WWF prior to placing concrete. WWF shall be continuously supported at 36" o.c. maximum. Reinforcing steel for slabs on grade shall be adequately supported on precast concrete units. Lifting the reinforcing off the grade during placement of concrete is not permitted.
   - b. Concrete to be mechanically consolidated during placement per ACI standards.
   - c. Contractor shall coordinate placement of all openings, curbs, dowels, sleeves, conduits, bolts, inserts and other embedded items prior to concrete placement.
   - d. All embeds and dowels shall be securely tied to formwork or to adjacent reinforcing prior to the placement of concrete.
   - e. No pipes, ducts, sleeves, etc shall be placed in structural concrete unless specifically detailed or approved by the structural engineer. Penetrations through walls when approved shall be built into the wall prior to concrete placement. Penetrations will not be allowed in footings or grade beams unless detailed. Piping shall be routed around these elements and footings stepped to avoid piping.
9. Detailing:
   - a. Lap lengths shall be as follows:
     - i. 30 bar diameters for #3 and #4 bars
     - ii. 40 bar diameters for #5 through #8 bars
     - iii. Do not splice stirrups and ties.
     - iv. Do not splice vertical bars in retaining walls unless specifically shown.
   - b. At joints provide reinforcing dowels to match the member reinforcing, unless noted otherwise.
   - c. At all discontinuous control or construction slab on grade joints, provide 2 - #4 x 48 inches.
   - d. Provide corner bars at intersecting wall corners using the same bar size and spacing as the horizontal wall reinforcing.
   - e. All vertical reinforcing shall be doweled to footings, or to the structure below with the same size and spacing as the vertical reinforcing for the element above. Dowels extending into footings shall terminate with a 90 degree standard hook and shall extend to within 4" of the bottom of the footing. Footing dowels (#8 bars and smaller) with hooks need not extend more than 20" into footings.
   - f. Horizontal wall reinforcing shall terminate at ends of walls and openings into the far end of the jamb column with a 90-degree standard hook plus a 6 bar diameter extension. Horizontal wall reinforcing shall be continuous through construction and control joints.

EPOXY

1. Epoxy shall be "HIT RE 500" by Hilti Corporation, "Anchor-It" by Adhesive Technology Corporation, "Epcon Injection System" by Ramset/Redhead, "Power-Fast" by Rawl, or approved equal.
2. All drilled holes shall be 1/8 inch larger than the bar or anchor bolt being installed.
3. After drilling the proper size hole, clean the walls and bottom of the hole of all dust and debris using a nylon brush in conjunction with oil free compressed air. The hole shall be free of dust, debris and standing water.
4. Follow all manufacturer's recommendations for epoxy installation.

MASONRY VENEER

1. Masonry veneer shall be attached to existing masonry with *Dur-O-Wal* DA 213 seismic veneer anchors or *Hohmann & Barnard* DW-10 or DW-10HS seismic veneer anchors (or equal) spaced at 16" o.c. Veneer anchors shall be attached to existing masonry with ¼ inch expansion bolt. Attach the veneer to the anchors with *Dur-O-Wal* Seismic Steel Pintles or *Hohmann & Barnard* 3/16" Ø Byna-Tie with Seismiclips (or equal) spaced at a maximum of 16" o.c. in both directions. Anchor ties shall engage to a galvanized No. 9 gauge horizontal joint reinforcement wire in the veneer, which shall be continuous and shall be placed at 16" o.c. maximum at the center of the veneer.
2. Other methods of attachment may be used after written acceptance by the architect and structural engineer.

STRUCTURAL STEEL

1. Material:
   - a. Wide Flanges Section — ASTM A992 (50 ksi)
   - b. Other shapes & Plates — ASTM A36
   - c. Pipe Columns — ASTM A53, Types E or S, Grade B.
   - d. Steel Tubes — ASTM A500 Grade B (46KSI)
   - e. Deformed Bar Anchors (DBA) — ASTM A496
   - f. Headed Stud Anchors (HSA) — ASTM A108
   - g. Anchor Rods
     - Gravity Columns — ASTM F1554, Grade 36, with ASTM A563 heavy hex nuts and hardened washers Grade A
   - h. Bolted Connections: — ASTM A325
2. Fabrication and construction shall comply with the latest edition of the following Codes and Standards:
   - a. American Institute of Steel Construction (AISC), "Specification for the Design, Fabrication and Erection of Structural Steel for Buildings," with "Commentary".
   - b. AISC "Code of Standard Practice" excluding the following: Section 3.4, Section 4.4, Section 4.4.1,
   - c. AISC "Specification for Structural Joints Using ASTM A325 or A490 Bolts"
   - d. American Welding Society (AWS), Structural Welding Code (specific items do not apply when they conflict with the AISC requirements).
   - e. AISC "Seismic Provision for Structural Steel Buildings"
3. Welding
   - a. All welding and cutting shall be performed by AWS certified welders.
   - b. Use E-70 XX or as noted otherwise. E60 XX may be used for welding steel roof decks.
   - c. All intersecting steel shapes which are not bolted shall be connected by a fillet weld all around, unless noted otherwise. Where fillet weld sizes are not shown they shall be 1/16" less than the thinnest of the connected parts for thicknesses 1/4" and larger. Fillet welds on plates less than 1/4" shall be of the same size as the thinnest of the connected part.
   - d. Reinforcing Bars: Do not weld rebar except as specifically detailed in the drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for deformed bar anchors (DBAs), machine bolts, or headed stud anchors(HSAs).
   - e. Do not weld anchor bolts, including "tack" welds.
   - f. Headed Stud Anchors (HSAs) welding and deformed bar anchor welding shall conform to the manufacturer's specifications.
4. Bolted Connections
   - a. Use ASTM A325N bolts for steel to steel connections, as noted herein or as noted on the drawings. A325N bolts shall be used in connections for simple span framing and beam (or girder) to bearing plate connections. Tighten bolts to a snug tight condition.
   - b. Use hardened washers beneath the turned element of all bolts or nuts. Use hardened beveled washers, to compensate for the lack of parallelism, where the outer face of the bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolt axis. At oversized holes hardened washers or plates shall conform with ASTM F-436 and shall completely cover the slot after installation.
   - c. Where a steel to steel beam connection is not shown, provide a standard AISC framed connection for one half the total uniform load capacity of the beam for the span and steel specified.
   - d. Bolts, nuts and washers shall not be reused.
5. Provide full-depth web-stiffener plates at each side of all beams at all bearing points. Stiffener plates shall be the thickness called out below unless noted otherwise and shall be welded both sides with fillet welds all around.

FLANGE WIDTH	STIFFENER THICKNESS	WELD SIZE
Less than 8 1/4"	1/4"	3/16"
8 1/4" to 12 1/4"	3/8"	1/4"
12 1/4" to 16 ½"	1/2"	5/16"
16 ½" to 20 ¾"	5/8"	3/8"

METAL DECKING

1. Steel deck shall comply with the latest requirements of the Steel Deck Institute.
2. All deck shall be 3-span continuous minimum. In areas where 3-span conditions are not possible, the contractor shall provide heavier gauge deck as required to provide the equivalent loading of the deck under a three span condition.
3. Steel roof deck shall not be used to support loads from plumbing, HVAC ducts, light fixtures, architectural elements or equipment of any kind, unless specifically noted.
4. All deck supporting members shall be dry before welding.
5. Crimp seams welding interlocking seams.
6. Yield stress of the 22 gauge steel deck shall be limited to a maximum of 50 ksi.

Steel Roof Deck

a. Steel roof deck shall be 1 1/2" deep X 20 gauge minimum painted, type "B" wide rib deck with interlocking side seams with the following properties:

	22 Gauge	20 Gauge	18 Gauge
Minimum S (in^3/ft) =	0.178	0.234	0.322
Minimum I (in^4/ft) =	0.172	0.213	0.296

b. Minimum allowable deck diaphragm shear values shall be 458 lb/ft. for a 7'-0" deck span.
c. Weld steel roof deck to supporting framing members with 3/4" diameter puddle welds at the following spacings (Closer spacing may be used to develop minimum shear requirements.):
   - i. 6" o.c. to all supports perpendicular to deck corrugations (7 welds per 36" sheet).
   - ii. 6" o.c. to all supports parallel to deck corrugations.
d. Attach interlocking seams with 1 1/2" long top seam welds at 24" o.c. maximum or with Verco PunchLok System at 24" o.c. maximum or with ASC Delta Grip System at 16" o.c. maximum. Closer spacing may be used to develop minimum shear requirements.
e. Provide a 2-inch minimum bearing and a 4-inch lap at the splice points.

SPECIAL INSPECTION AND QUALITY ASSURANCE

Special inspection and quality assurance, as required by section 1704 of the IBC, shall be provided by an independent agency employed by the owner unless waived by the building official. The contractor shall coordinate and cooperate with the required inspections. All testing and inspection reports shall be sent to the engineer for review. Items requiring special inspection and quality assurance are:

1. Structural welding, including steel deck (IBC 1704.3)
   - a. Periodic special inspection of metal floor and roof decks.
   - b. Periodic special inspection of single pass fillet welds less than or equal to 5/16"
   - c. Continuous special inspection of single pass fillet welds greater than 5/16" and multi-pass fillet welds.
   - d. Continuous special inspection of complete and partial penetration welds.

DEFERRED SUBMITTALS

For the purpose of this section, deferred submittals are defined as per section 106.3.4.2 of the IBC. Submittal documents for deferred submittal items shall be submitted to the engineer/architect for their review for general conformance with the design of the building. Deferred structural submittals for this project are:

1. None

LEGEND OF MARKS AND ABREVIATIONS

AB	ANCHOR BOLT(S)
ABV	ABOVE
ALT	ALTERNATE
APPROX	APPROXIMATE
ARCH	ARCHITECT(URAL)
BLDG	BUILDING
BLW	BELOW
BM	BEAM
BOT	BOTTOM
BRG	BEARING
BTWN	BETWEEN
CC	CENTER-TO CENTER
CJ	CONSTR.CONTROL JOINT
COL	COLUMN
CONC	CONCRETE
CONST	CONSTRUCTION
CTR	CENTER
CW-x	CONCRETE WALL
DB	DECK BEARING
DBA	DEFORMED BAR ANCHOR
DBE	DECK BEARING ELEVATION
DBL	DOUBLE
DET	DETAIL
DIA	DIAMETER
DIM	DIMENSION
DN	DOWN
DWG	DRAWING
DWL	DOWEL
EA	EACH
E.F.	EACH FACE
E.J.	EXPANSION JOINT
ELEC	ELECTRICAL
ELEV	ELEVATION
EQUIP	EQUIPMENT
EQ	EQUAL
E.W.	EACH WAY
EXST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
FC-x	CONTINUOUS FOOTING MARK
F.D.	FLOOR DRAIN
FDN	FOUNDATION
F.F.	FINISHED FLOOR
FR-x	RECTANGULAR FOOTING MARK
FS-x	SQUARE FOOTING MARK
FT	FOOT
FTG	FOOTING
FTS-x	THICKEN SLAB MARK
GA	GAUGE
GALV	GALVANIZED
GSN	GENERAL STRUCTURAL NOTES
HB	HORIZONTAL BRIDGING
HORIZ	HORIZONTAL
HSA	HEADED STUD ANCHOR
HT	HEIGHT
ICBO	INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS
IBC	INTERNATIONAL BUILDING CODE
I.F.	INSIDE FACE
IN.	INCH
INT	INTERIOR
JT	JOINT
JST	JOIST
k	KIP(S) = 1000 POUNDS
KLF	KIPS PER LINEAL FOOT
KSF	KIPS PER SQUARE FOOT
LBS	POUNDS
LF	LINEAL FOOT
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LSV	LONG SIDE VERTICAL
MAX	MAXIMUM
MCJ	MASONRY CONTROL JOINT
MC-x	MASONRY COLUMN MARK
MECH	MECHANICAL
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
NIC	NOT IN CONTRACT
NTS	NOT TO SCALE
O.C.	ON CENTER
O.F.	OUTSIDE FACE
OPNG	OPENING
OPP	OPPOSITE
PCF	POUNDS PER CUBIC FOOT
PL	PLATE
PLF	POUNDS PER LINEAL FOOT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	POINT
REINF	REINFORCING
REQD	REQUIRED
R.D.	ROOF DRAIN
RTU	ROOF TOP UNITS
SBP-x	STEEL BASE PLATE MARK
SC-x	STEEL COLUMN MARK
SCP-x	STEEL CAP PLATE MARK
SHT	SHEET
SI	SPECIAL INSPECTION
SIM	SIMILAR
SMU	SUSPENDED MECHANICAL UNITS
SOG	SLAB-ON-GRADE
SQ	SQUARE
STAG	STAGGERED
STD	STANDARD
STL	STEEL
STR	STRUCTURAL
STS	SELF TAPPING SCREWS
T&B	TOP AND BOTTOM
TEMP	TEMPERATURE
THDS	THREADS
T.O.	TOP OF
TOC	TOP OF CONCRETE
TOD	TOP OF DECK
TOF	TOP OF FOOTING
TOS	TOP OF STEEL
TOW	TOP OF WALL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W/	WITH
WWF	WELDED WIRE FABRIC
WWM	WELDED WIRE MESH

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SHEET TITLE

GENERAL
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NOTES

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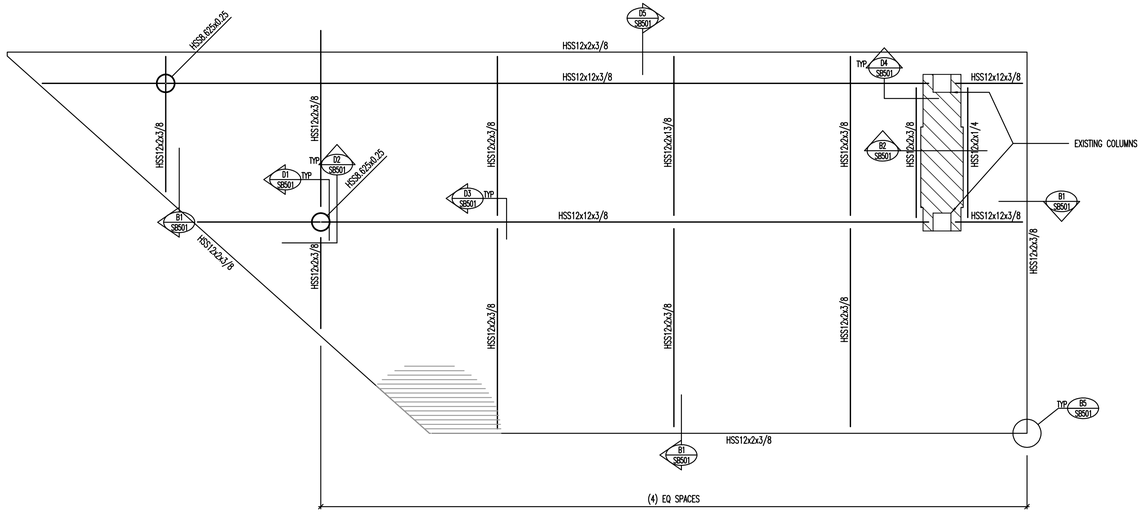
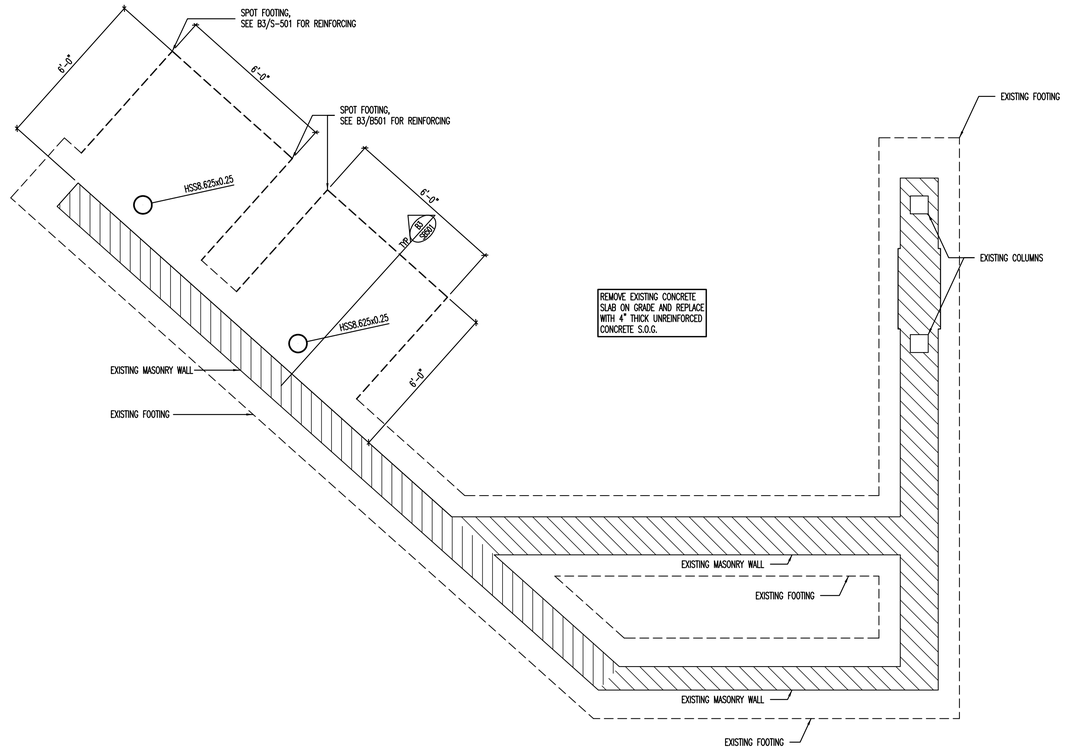
SJ001



ROOF FRAMING PLAN

SCALE: 3/8"=1'-0"



FOOTING AND FOUNDATION PLAN

SCALE: 3/8"=1'-0"

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MASONRY S-NOTE-ROOF

MARKS AND SYMBOLS LEGEND

- SECTION MARK / SHEET NUMBER
- INDICATES METAL ROOF DECK. SEE GENERAL STRUCTURAL ON SHEET SJ001.
- INDICATES CONCRETE WALL.
- INDICATES EXISTING MASONRY WALL.

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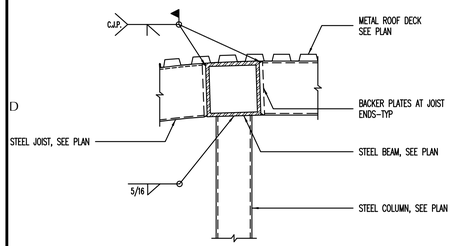
SHEET TITLE

FOOTING, FOUNDATION AND ROOF PLANS

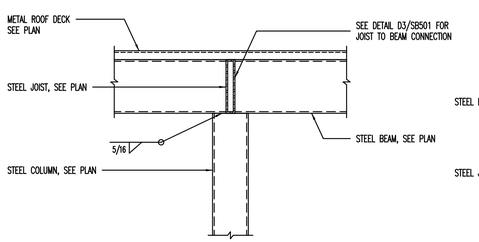
SB101



NOTE: SEE ARCH FOR BEAM RADIUS.

D1 COLUMN CONNECTION DETAIL

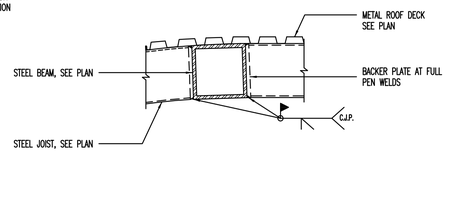
NO SCALE



NOTE: SEE ARCH FOR BEAM RADIUS.

D2 COLUMN CONNECTION DETAIL

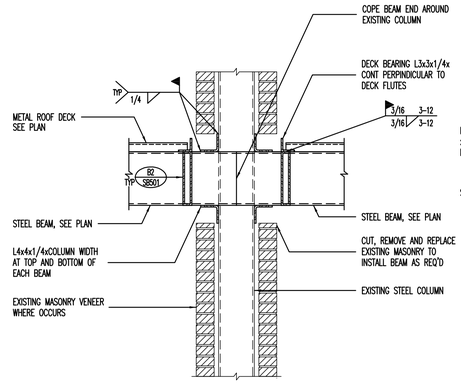
NO SCALE



NOTE: SEE ARCH FOR BEAM RADIUS.

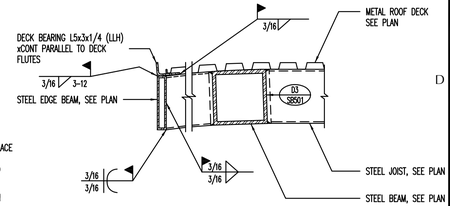
D3 JOIST TO BEAM DETAIL

NO SCALE



D4 BEAM TO EXISTING COLUMN

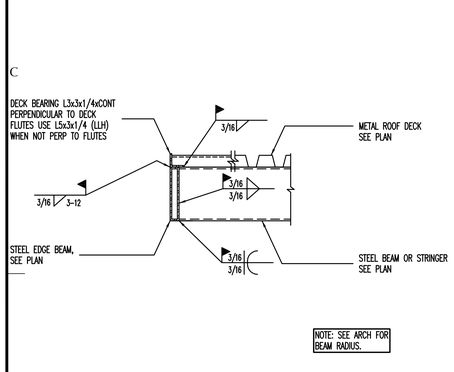
NO SCALE



NOTE: SEE ARCH FOR BEAM RADIUS.

D5 ROOF DECK EDGE DETAIL

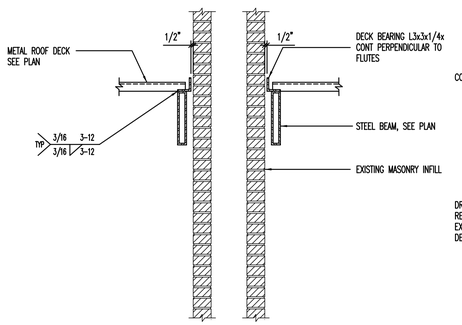
NO SCALE



NOTE: SEE ARCH FOR BEAM RADIUS.

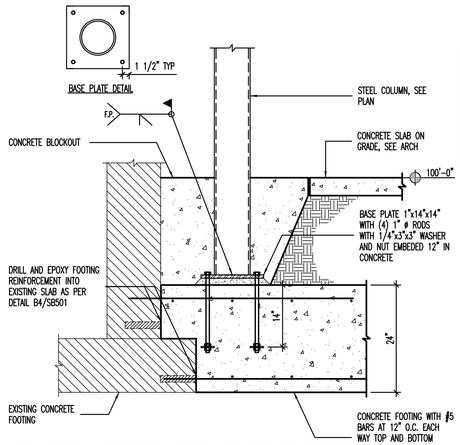
B1 DECK BEARING DETAIL

NO SCALE



B2 DECK BEARING AT EXISTING SIGN

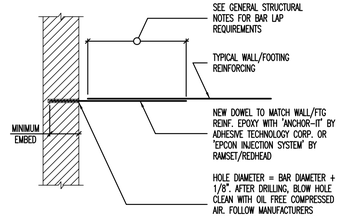
NO SCALE



B3 TYPICAL INTERIOR STEEL COLUMN TO FOOTING DETAIL

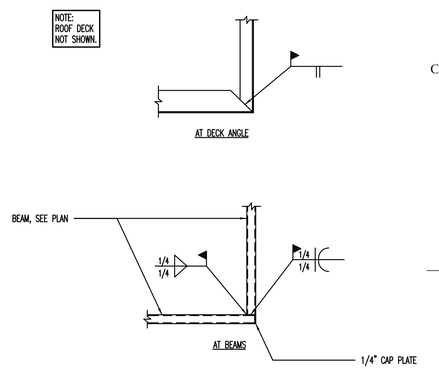
NO SCALE

EPOXY DOWEL EMBED SCHEDULE	
DOWEL SIZE	MINIMUM EMBEDMENT INTO EXISTING CONCRETE
#4	6 1/2"
#5	7 1/2"
#6	10"
#7	1'-1"
#8	1'-4"



B4 EPOXY DOWEL EMBED SCHEDULE

NO SCALE



B5 TYPICAL CORNER DETAIL

NO SCALE

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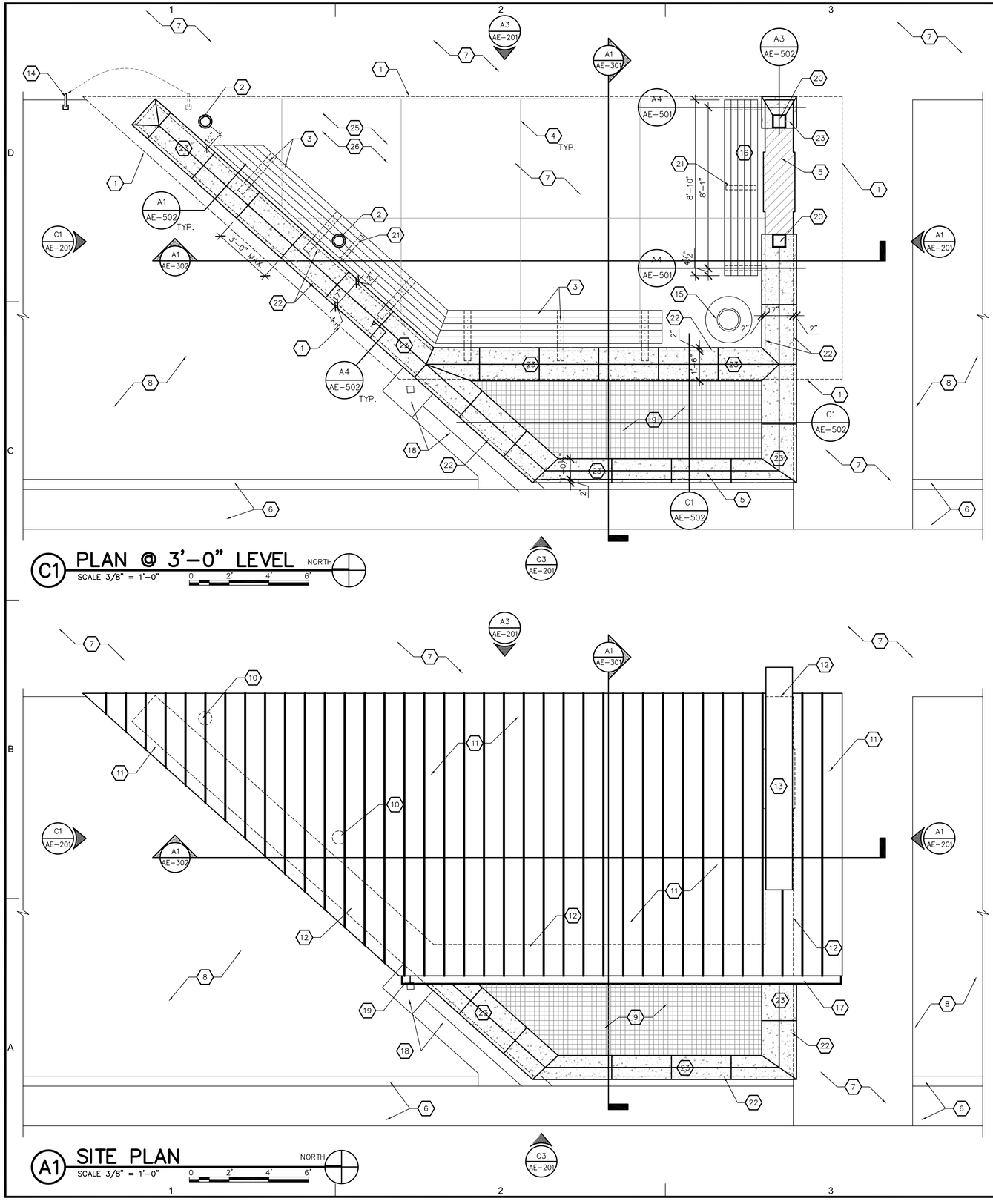
SHEET TITLE

FOOTING AND ROOF DETAILS

SB501



C1 PLAN @ 3'-0" LEVEL
SCALE 3/8" = 1'-0"
NORTH
0 2' 4' 6'

A1 SITE PLAN
SCALE 3/8" = 1'-0"
NORTH
0 2' 4' 6'

KEYED NOTES

1. LINE OF NEW ROOF ABOVE SHOWN DASHED.
2. NEW STEEL 8" COLUMN – SCRIBE WOOD AROUND NEW COLUMN – SEE STRUCTURAL DRAWINGS.
3. REMOVE EXTG.. 4X4 WOOD BENCH PROVIDE NEW 4X4 REDWOOD BENCH BULLNOSE 3/4" MIN.. ON ALL EXPOSED EDGES ON EXTG.. STEEL STRUCTURE – PAINT EXTG.. STEEL STRUCTURE – SEE SPEC.
4. EXTG.. JOINTS IN CONCRETE TYP..
5. EXTG.. BRICK WALL TO REMAIN – PROTECT FROM DAMAGE DURING CONSTRUCTION.
6. EXTG.. CURB & GUTTER TO REMAIN – PROTECT FROM DAMAGE DURING CONSTRUCTION.
7. EXTG.. SIDEWALK & PLAZA TO REMAIN – PROTECT FROM DAMAGE DURING CONSTRUCTION.
8. EXTG.. LANDSCAPING TO REMAIN – PROTECT FROM DAMAGE DURING CONSTRUCTION.
9. EXTG.. STEEL GRATE TO REMAIN – PROTECT FROM DAMAGE DURING CONSTRUCTION – PAINT.
10. NEW STEEL 8" COLUMN BELOW SHOWN DASHED – SEE STRUCTURAL DRAWINGS.
11. NEW STANDING SEAM METAL ROOF SYSTEM – SEE DETAIL INDICATED & SEE STRUCTURAL DRAWINGS.
12. EXTG.. BRICK WALL BELOW SHOWN DASHED TO REMAIN – PROTECT FROM DAMAGE DURING CONSTRUCTION.
13. EXTG.. ELECTRONIC SIGN.
14. RELOCATE EXTG.. BUS STOP SIGN TO THIS LOCATION.
15. NEW LOCATION OF EXTG.. TRASH CONTAINER.
16. NEW 4X4 REDWOOD BENCH TO MATCH EXTG.. SEE DETAIL INDICATED.
17. GUTTER WITH CHAIN (GALV..) DOWN–SPOUT TO SPLASH BLOCK BELOW.
18. 6" DEEP CUSTOM CONCRETE SPLASH BLOCK TO GUTTER.
19. OPENING IN GUTTER WITH CHAIN TO SPLASH BLOCK.
20. PAINT EXTG.. STEEL COLUMNS TO MATCH NEW COLUMNS TYP..
21. NEW 1/8"X2-1/2" FLAT BAR STEEL STRAP ANCHOR WITH 1-1/2" LONG WOOD LAG SCREWS IN EACH WOOD MEMBER ON BOTTOM.
22. DASHED LINE INDICATES EXTG.. MASONRY WALL BELOW – CLEAN ALL EXTG.. MASONRY AS REQUIRED – SEE SPEC..
23. NEW PRECAST CONCRETE CAP 4,000 PSI WITH INTEGRATED DRIP – REFER TO DETAIL INDICATED – PROVIDE SHOP DRAWINGS.
24. REMOVE EXTG.. CONCRETE FOR CONSTRUCTION OF NEW FOOTING & COLUMN – REMOVE TO NEAREST EXTG.. JOINTS – SAW CUT TO PREVENT EDGE BREAKAGE.
25. NEW CONCRETE – PROVIDE 3/4" MAX.. GRAVEL UNDER ALL NEW CONCRETE.

GENERAL NOTES

1. FIELD VERIFY ALL DIMENSION.
2. PAINT EXTG. STEEL COLUMNS AND ALL EXTG. EXPOSED STEEL AND NEW STEEL AS REQUIRED. INCLUDING BUT NOT LIMITED TO THE STEEL GRATE OVER THE ELECTRICAL EQUIPMENT AREA, STEEL BRACKETS AT BENCHES AND ALL ELECTRICAL STEEL COVER PLATES ETC.
3. PROVIDE SHOP DRAWINGS FOR NEW PRECAST CONCRETE WALL CAP – SEE PLAN AND DETAILS.

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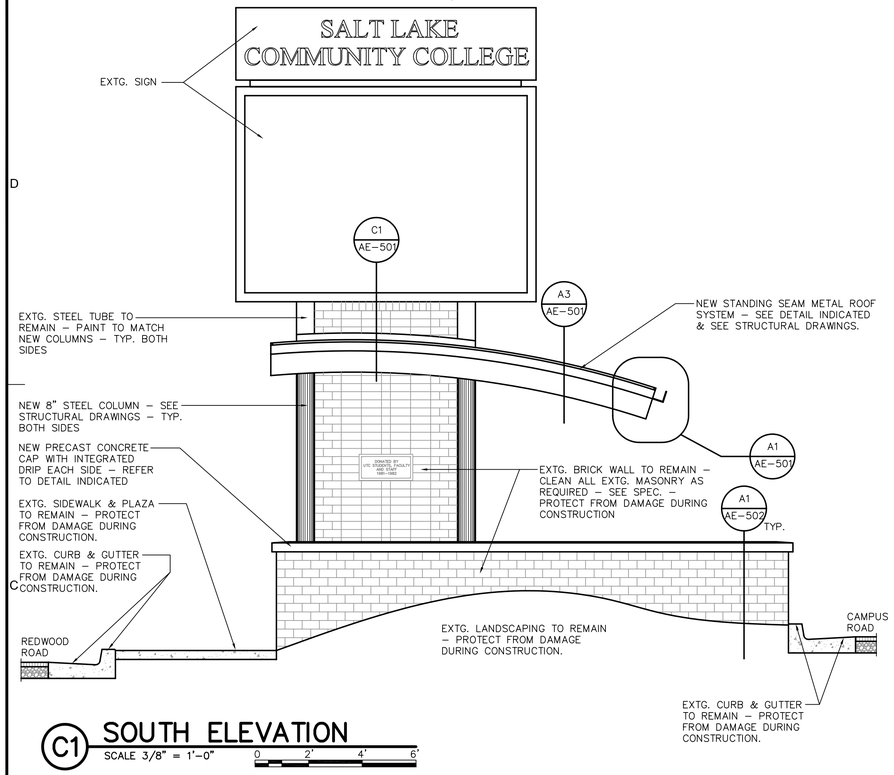
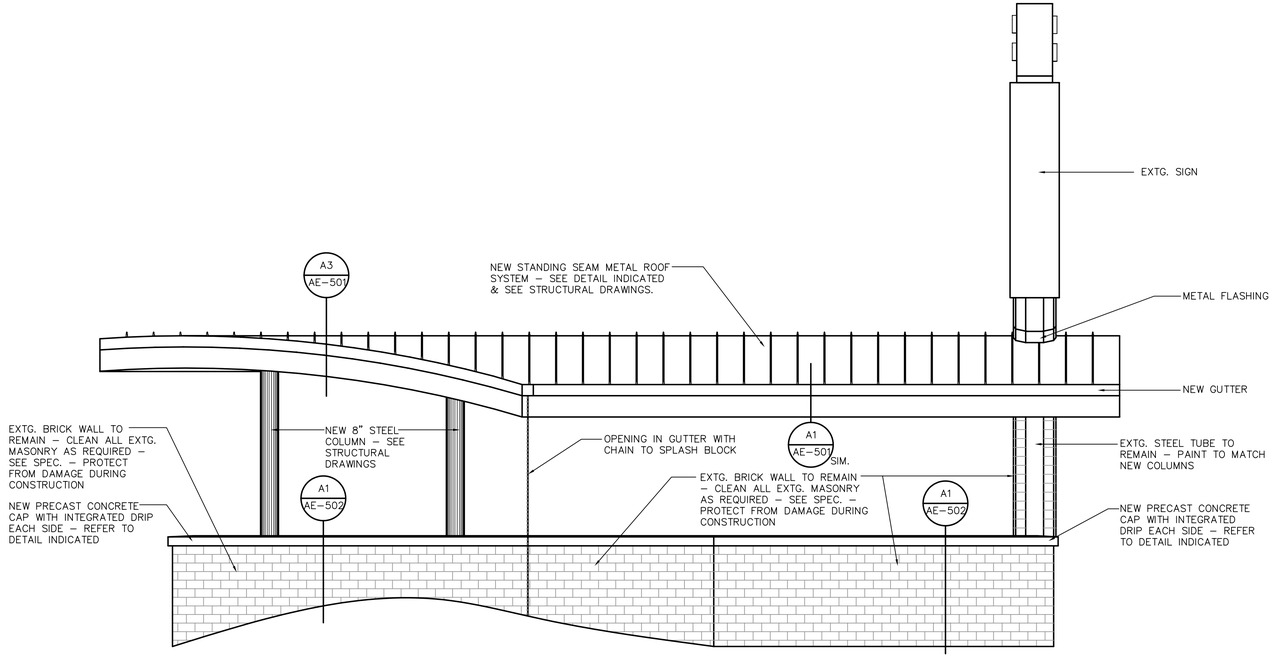
SHEET TITLE
PLANS & KEYED NOTES

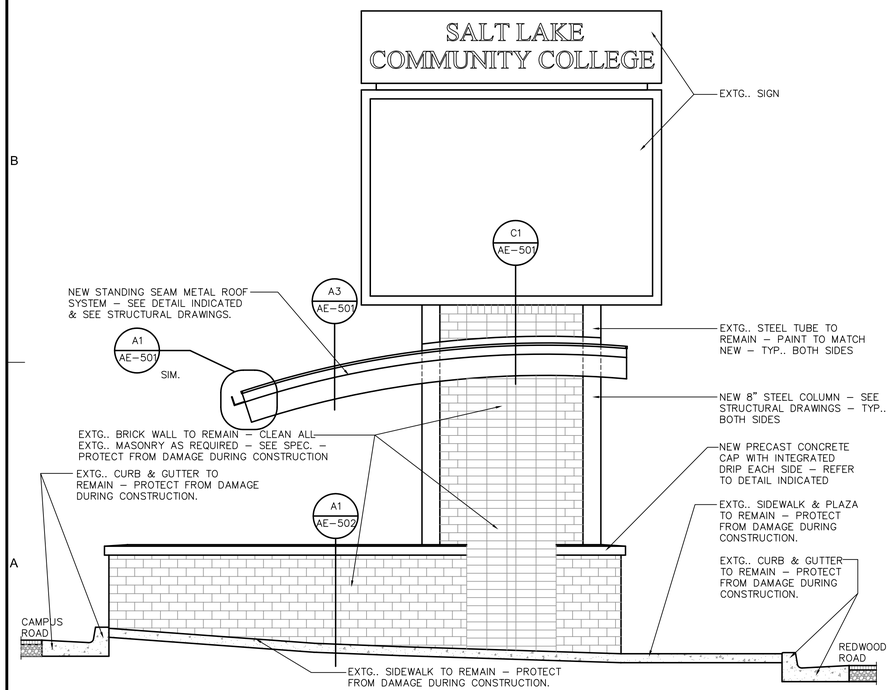
AE-101



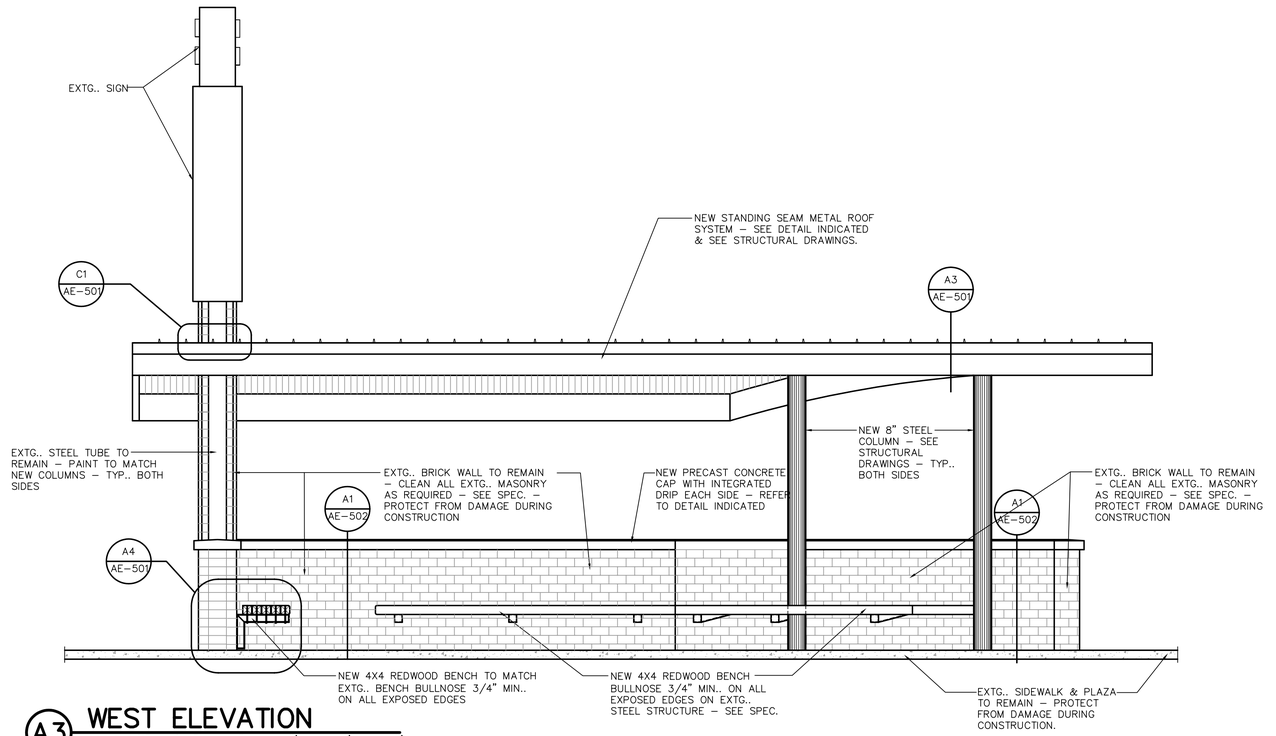
C1 SOUTH ELEVATION
SCALE 3/8" = 1'-0"



C3 EAST ELEVATION
SCALE 3/8" = 1'-0"



A1 NORTH ELEVATION
SCALE 3/8" = 1'-0"



A3 WEST ELEVATION
SCALE 3/8" = 1'-0"

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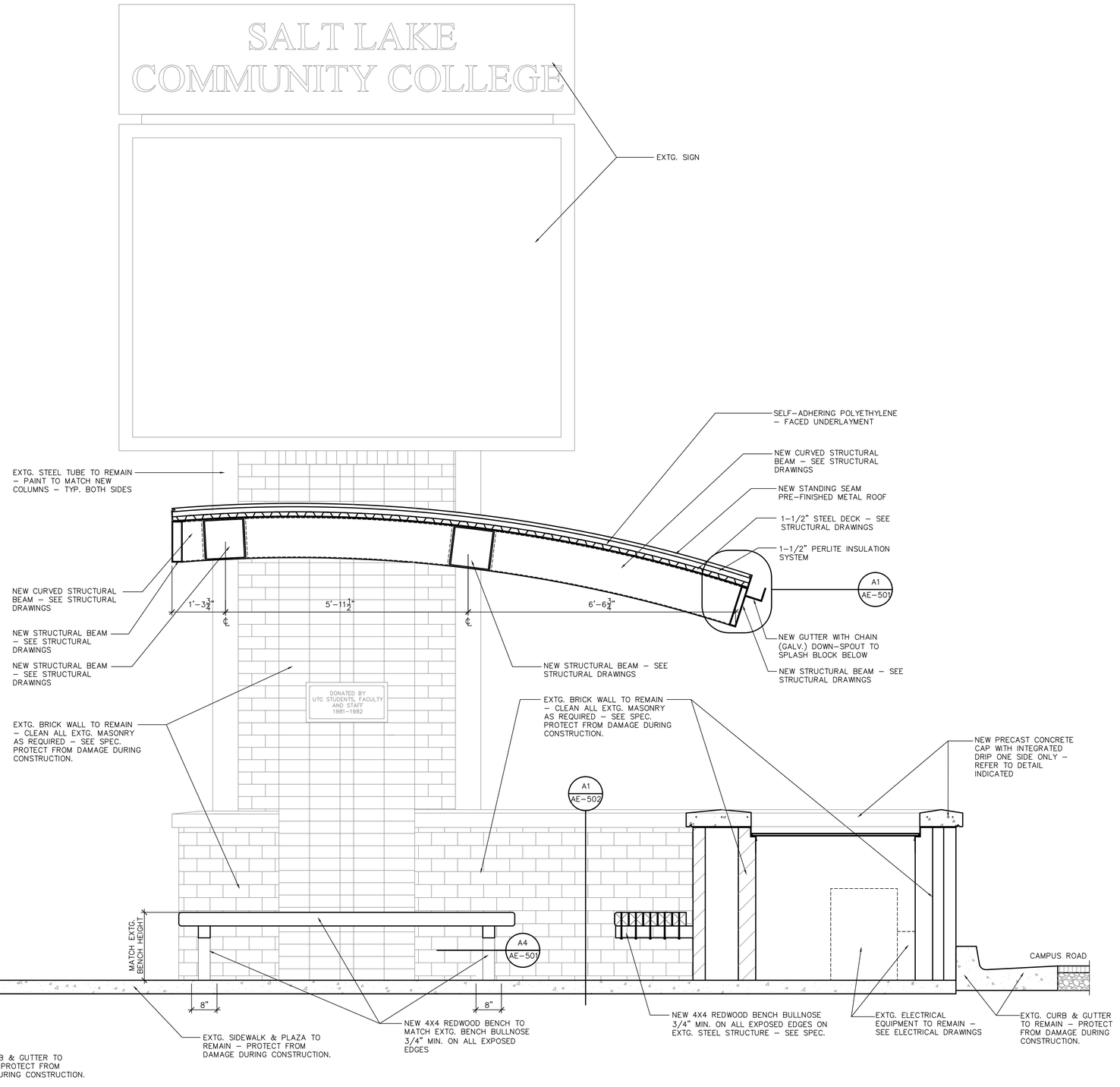
SHEET TITLE
EXTERIOR ELEVATIONS

AE-201



A1 SECTION

SCALE 3/4" = 1'–0"

0 1' 2' 3'

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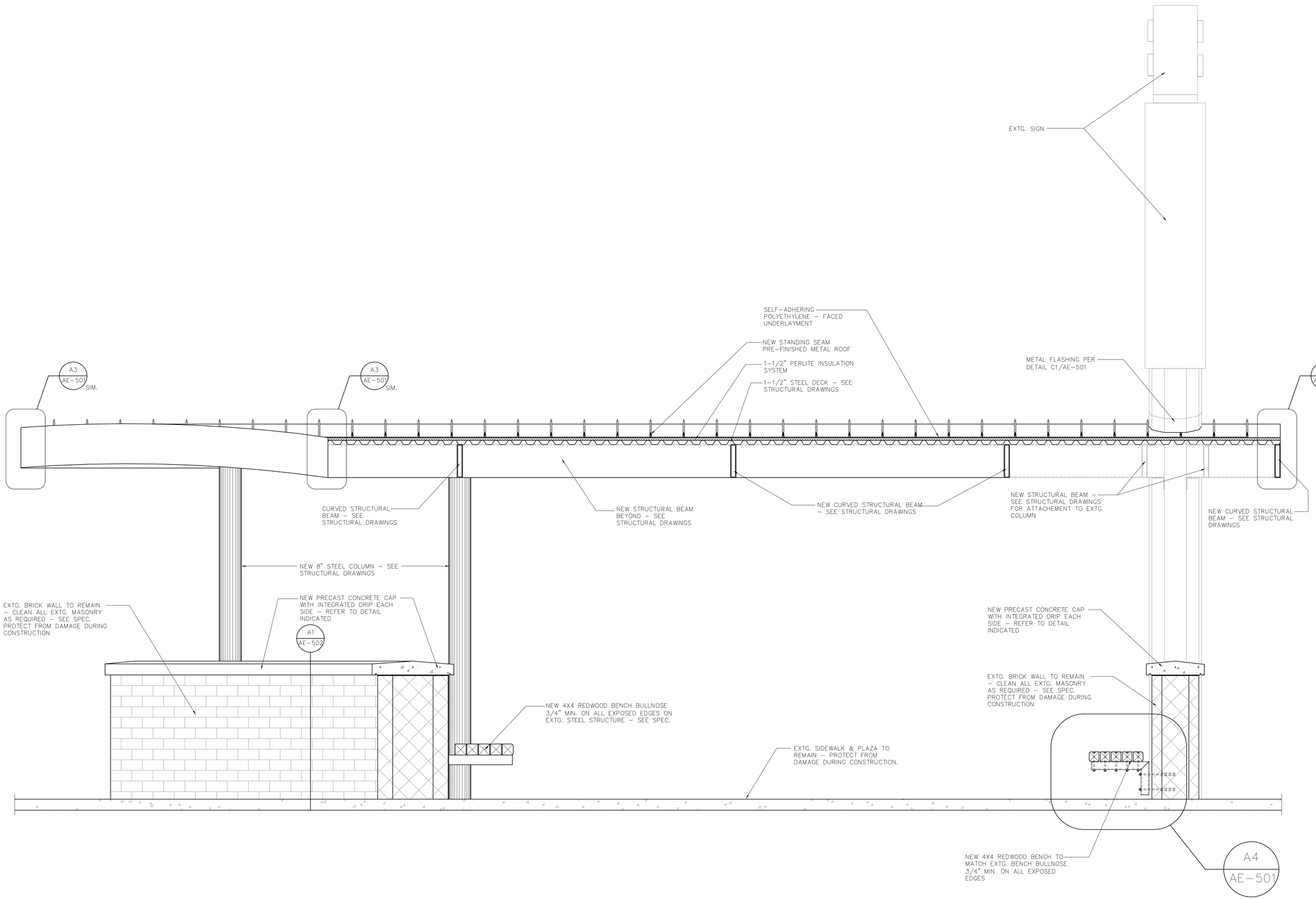
SHEET TITLE

SECTION

AE-301



A1 SECTION

SCALE 3/4" = 1'-0"

0 1' 2' 3'

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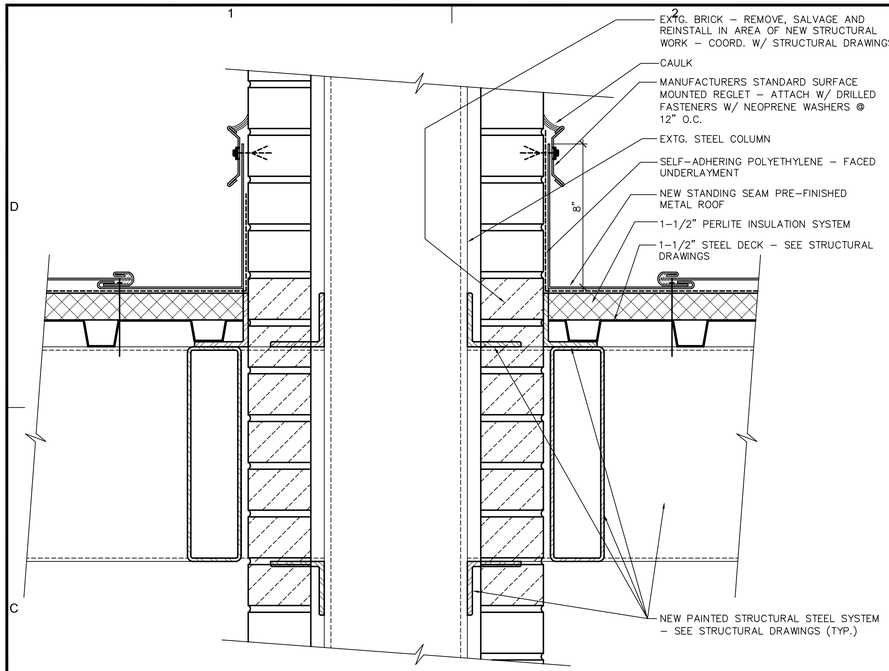
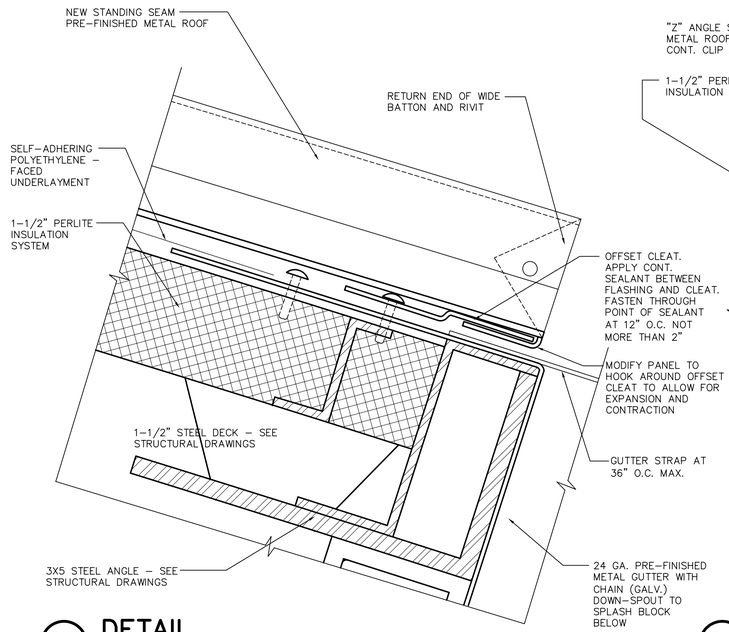
SHEET TITLE

SECTION

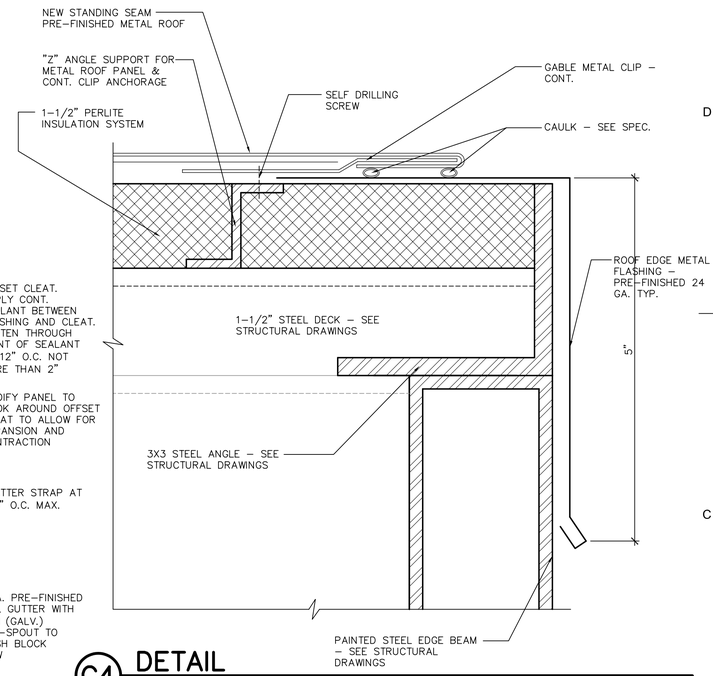
AE-302



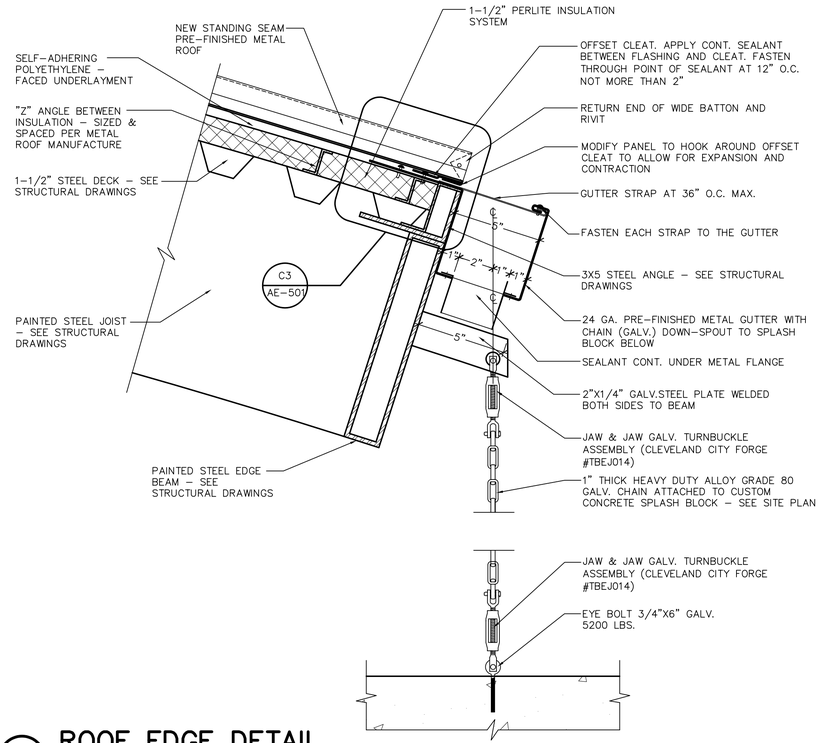
C1 ROOF TO WALL TRANSITION
SCALE 3" = 1'–0"



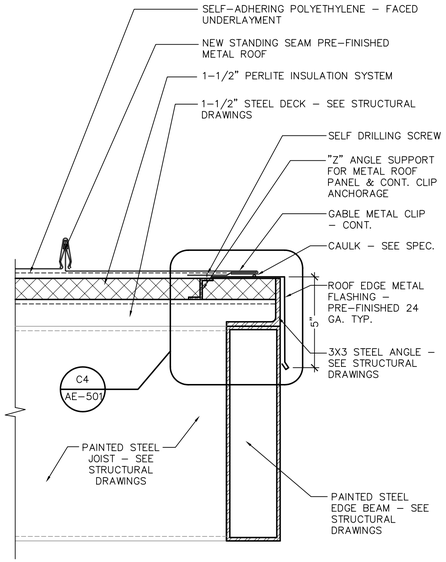
C3 DETAIL
SCALE 1" = 1"



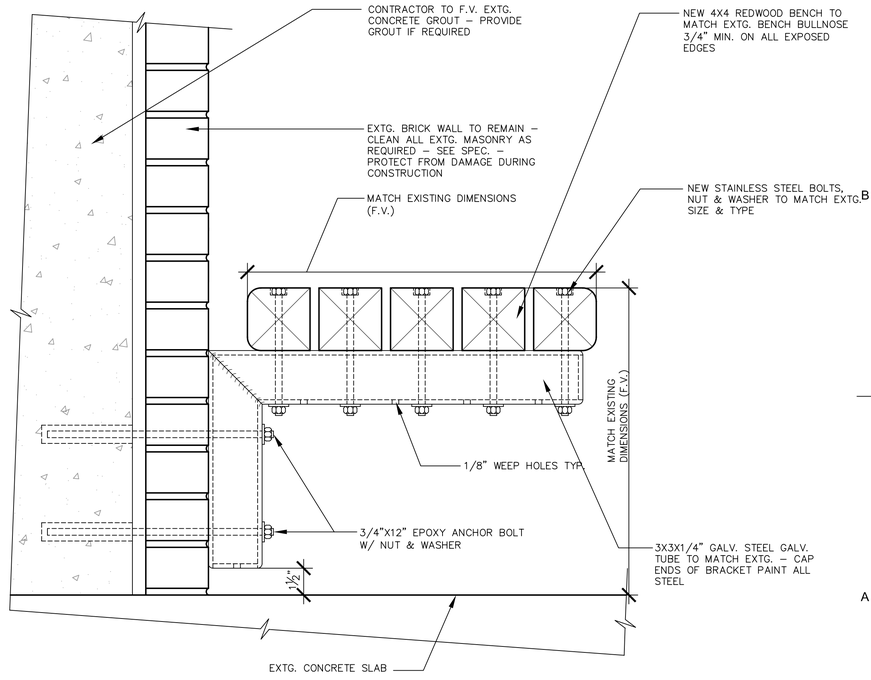
C4 DETAIL
SCALE 1" = 1"



A1 ROOF EDGE DETAIL
SCALE 3" = 1'–0"



A3 ROOF EDGE DETAIL
SCALE 3" = 1'–0"



A4 NEW BENCH DETAIL
SCALE 3" = 1'–0"

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REDWOOD ROAD CAMPUS
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MARK	DATE	DESCRIPTION
	10.11.06	CONSTRUCTION DOCUMENTS

DFCM PROJECT NO:	06305660
ARCH. PROJECT NO:	05-08
CAD DWG FILE:	AE-501.DWG
DRAWN BY:	JBE
CHECKED BY:	SPE
DESIGNED BY:	SPE
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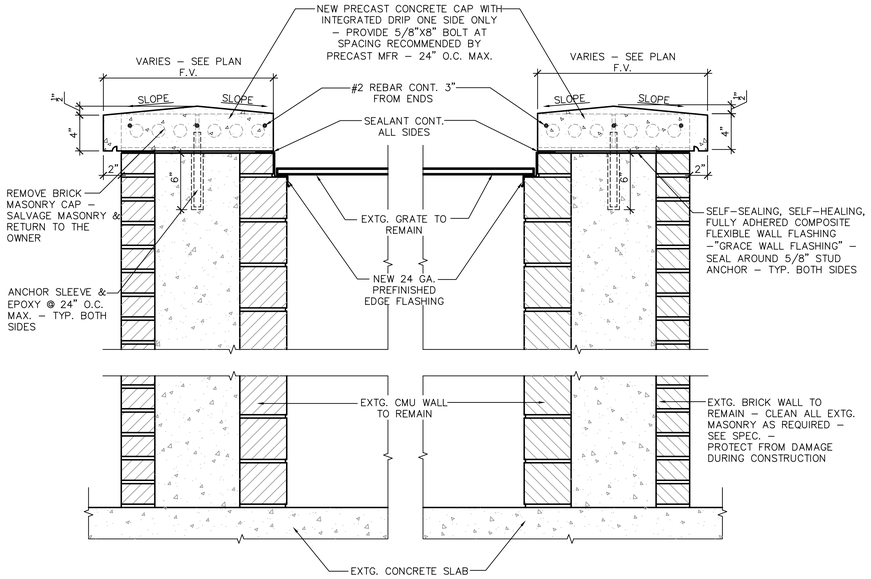
SHEET TITLE
DETAILS

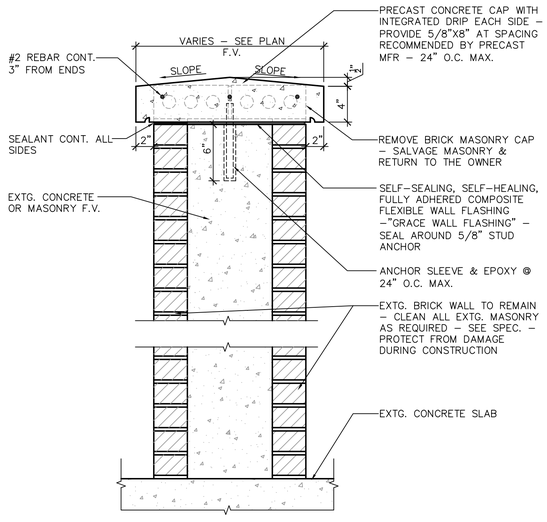
AE-501



C1 PRECAST CONCRETE CAP DETAIL

SCALE 1 1/2" = 1'–0"

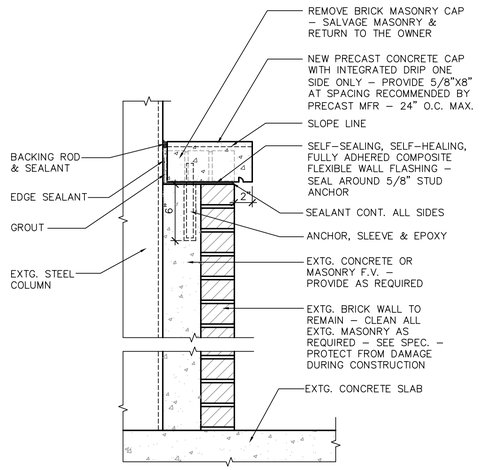
0 1/2' 1' 1 1/2'



A1 PRECAST CONCRETE CAP DETAIL

SCALE 1 1/2" = 1'–0"

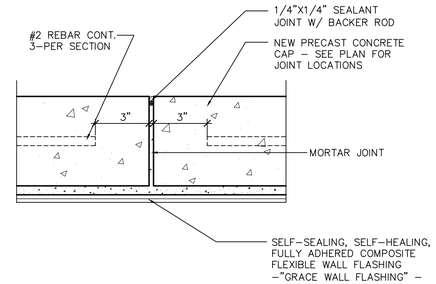
0 1/2' 1' 1 1/2'



A3 PRECAST CONCRETE CAP DETAIL

SCALE 1 1/2" = 1'–0"

0 1/2' 1' 1 1/2'



A4 JOINT DETAIL

SCALE 3" = 1'–0"

0 3" 6" 9"

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	10.11.06	CONSTRUCTION DOCUMENTS

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CAD DWG FILE:	AE-502.DWG
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DESIGNED BY:	SPE

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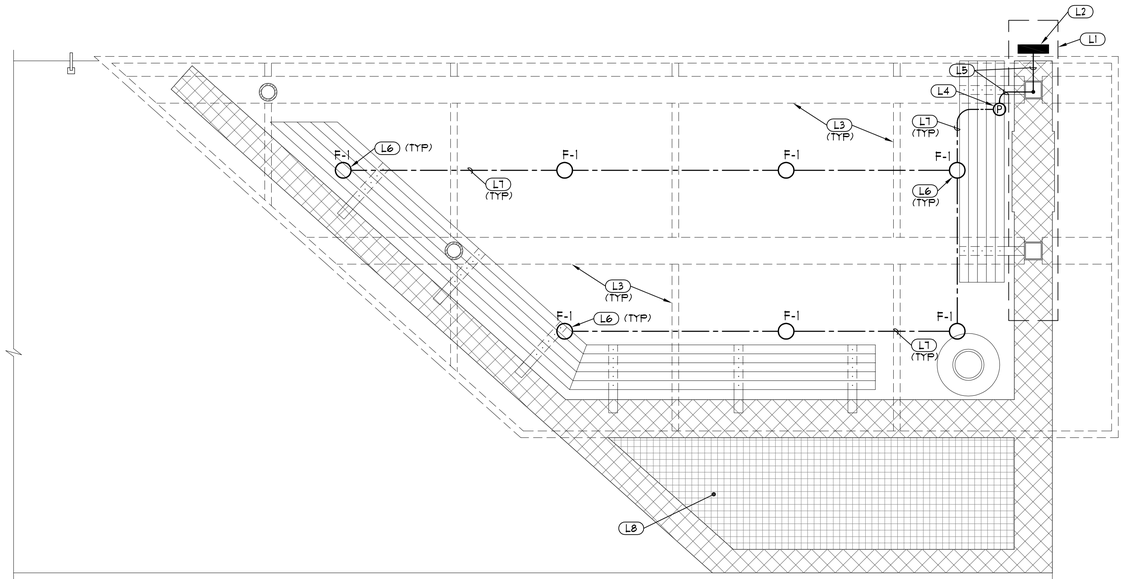
SHEET TITLE

DETAILS

AE-502



C2 ELECTRICAL LIGHTING PLAN
SCALE: 3/8" = 1'-0"

NORTH
0 1' 2' 4' 6'

GENERAL NOTES:

1. LOCATIONS OF EXISTING ELECTRICAL EQUIPMENT, OUTLETS, FEEDERS, BRANCH CIRCUIT WIRING, ETC., ARE BASED ON FIELD OBSERVATION OF EXISTING SURFACE CONDITIONS. FIELD VERIFY EXISTING LOCATIONS AND CIRCUITING AND IMMEDIATELY NOTIFY THE ARCHITECT OF ANY DISCREPANCIES WHICH MAY ADVERSELY AFFECT COMPLETION OF THE WORK.
2. TAKE ALL PRECAUTIONS NECESSARY TO AVOID DAMAGE TO THE EXISTING STRUCTURE AND SIGN. REPAIR ALL DAMAGE INCURRED BY NEW CONSTRUCTION TO EXACTLY MATCH SURROUNDING SURFACES AND/OR CONDITIONS WITHOUT ADDITIONAL COST TO THE OWNER. COORDINATE REPAIRS WITH THE GENERAL CONTRACTOR.
3. PROVIDE NEW IP-20A BREAKERS IN EXISTING SIGN ELECTRICAL PANEL AS REQUIRED FOR CONNECTION TO NEW LIGHTING.
4. COORDINATE ALL ELECTRICAL CONNECTIONS TO SIGN WITH OWNER, GENERAL CONTRACTOR, SIGN CONTRACTOR AND ARCHITECT.

LIGHTING PLAN KEYED NOTES:

(L1) OUTLINE OF EXISTING SIGN ABOVE.

(L2) EXISTING ELECTRICAL PANEL, 100 AMPERE, 120/208 V, 3Φ, 4W, IN EXISTING SIGN.

(L3) LINE OF EXPOSED BEAMS IN NEW CANOPY. SEE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR DETAILS. (TYPICAL)

(L4) PROVIDE NEW PHOTOCELL, TORK #101, OR EQUAL RATED 2000 WATTS, 120 VOLT. PROVIDE CAST METAL OUTLET BOX SECURED TO NEW BEAM FOR PHOTOCELL. AIM PHOTOCELL AWAY FROM NEW LIGHT FIXTURES.

(L5) PROVIDE TYPE 'MC' METAL CLAD CABLE FROM NEW PHOTOCELL OUTLET BOX THROUGH NEW BEAM AND EXISTING COLUMN TO EXISTING SIGN ELECTRICAL PANEL. COORDINATE INSTALLATION WITH GENERAL CONTRACTOR.

(L6) SURFACE MOUNT NEW FIXTURE TO BOTTOM OF METAL ROOF DECK. PROVIDE SURFACE MOUNTED CAST METAL OUTLET BOX AS REQUIRED FOR FIXTURE MOUNTING. COORDINATE LOCATIONS WITH EXPOSED CANOPY STRUCTURE.

(L7) PROVIDE EMT CONDUIT EXPOSED ON BOTTOM OF METAL ROOF DECK. OFFSET CONDUIT TO ABOVE BEAMS AND THROUGH METAL ROOF DECK WHERE REQUIRED. COORDINATE INSTALLATION WITH GENERAL CONTRACTOR.

(L8) EXISTING ELECTRICAL EQUIPMENT IN EXISTING AREA WELL WITH STEEL GRATING TO REMAIN.

FIXTURE SCHEDULE

SYMBOL	MANUFACTURER	CATALOG NUMBER	DESCRIPTION	LAMP
F-1	DAY-BRITE LUMARK SPAULDING	DSS-100-M-12-FGC MHCL-100-120-C73 HT1-CM-H10-FGP-Q-WH	SURFACE MOUNTED METAL HALIDE CANOPY FIXTURE WITH ALUMINUM HOUSING, WHITE FINISH, CORNING 'C73', OR EQUAL, FLAT PRISMATIC TEMPERED GLASS LENS, AND 120 VOLT HIGH POWER FACTOR BALLAST.	100 W MH ED-17

SYMBOL LIST

SYMBOL	DESCRIPTION
○	NEW CEILING MOUNTED FIXTURE
(P)	NEW PHOTOCELL
(J)	NEW JUNCTION BOX
▬	NEW POWER PANELBOARD, 120/208 VOLT, 3 PHASE
⌒	NEW BRANCH CIRCUIT CONCEALED IN WALL OR CEILING
⌒	NEW BRANCH CIRCUIT CONCEALED IN FLOOR OR WALL
⌒	NEW BRANCH CIRCUIT EXPOSED ON WALL OR CEILING
⌒	EXISTING BRANCH CIRCUIT
F-1	FIXTURE SCHEDULE SYMBOL
(L2)	KEYED NOTE SYMBOL
WP	INDICATES ITEM IN WEATHERPROOF (NEMA 3R MINIMUM) ENCLOSURE

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DESIGNED BY: WBG
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SHEET TITLE
ELECTRICAL PLANS
AND KEYED NOTES

E-101